# Ridgefield Pits Restoration Design: Physical Model Overview



#### Model Package: Tuflow (University of Queensland / British Maritime Technology)

#### **TUFLOW Software Areas of Application**



#### Catchments



#### Urban drainage and stormwater

Superior 1D solver for simulating structures provide the solution networks and over systems.



#### Floodplains and rivers



Where rivers and the sea meet: a complex interaction of tides, inflows and ocean currents combined with sediments and environmental issues



#### Coastal and nearshore

Winds and waves, hurricanes, wave setup and current generation.

Longshore transport of sediments and morphological change.

Tsunami propagation and mundation



#### Offshore and metocean

Providing a link between oceanography and coastal engineering with high resolution nesting and advanced 3D boundary

Supporting the oil and gas industry: oil spill response and drill mud dispersion

#### **Our History**



Deschwarked and unicted for Londa



TUFFOW Elastic dynamically freed to Good Modeller (CH2N) and



First integrated Livings Drainings III underground page network



FEMA (lingion 3 Approval and XP-20 (TUFLOW)

2010



TURLOW Classic/Pi/GPU perform successfully against all tests for the





Dity of See Yorkey FEMA



1989 1990

2002 2004 2006

2011 2012

2013

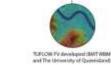
2014

2015 2016 2017

















Corps of Engineers "List of Approved

H&H Software







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#### **Tuflow FV Flexible Mesh (www.tuflow.com)**

#### Flexible Mesh Modelling

A 1D / 2D / 3D flexible mesh finite volume numerical model that simulates hydrodynamic, sediment transport and water quality processes in oceans, coastal waters, estuaries and rivers. A powerful engine proven at all scales.

#### V Solution Scheme

The finite volume numerical scheme solves the conservative integral form of the non linear shallow water equations (NLSWE).

The equations can be solved in 1D, 2D (vertically averaged) and 3D.

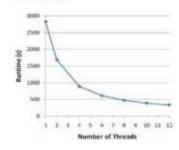
#### ngines

#### Key FV Scheme Features

- · Intrinsically handles shocks
- Subcritical, supercritical and transitional flows
- . Locally (and globally) conservative to numerical precision
- Robust wetting/drying
- Parallelised explicit scheme (varying Courant dependent timestep)
- . 2D and 3D with 1st and 2nd order schemes

#### Speed

TUFLOW FV code is optimised and parallelised for multi-processor machines, using the OpenMP implementation of shared memory parallelism.



#### Features

#### Flexible controls / interfacing

TUFLOW FV inputs are controlled via a macro style text-file interface. This allows the user to flexibly and efficiently control model configuration, boundary condition specification and output requirements.

#### Open boundaries

- Specified water level
- Specified discharge
- an circulation models

- Global cell inflows and outflows (e.g. rainfall, evaporation)
- . Cell inflows/outflows (e.g. pollutant source/sinks)
- atmospheric pressure
- · Holland parametric cyclone wind and pressure model

#### Structures

- · Weirs, culverts
- Adjustable beds, levee failure, etc.
- . hQh matrix specification at selected cells

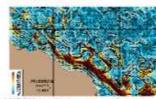
#### A combined 2D/3D flood model

TUFLOW FV has an option to insert 3D within a 2D model. This feature, which allows simulation of overland inundation plus complex 3D flow patterns in the main river channel, was successfully applied to the design of riverbank infrastructure.

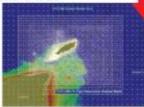
#### Complex problems require clever solutions

A nested modelling approach around Trinidad and Tobago to establish design parameters for infrastructure:

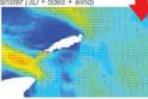
- . 3D currents and vortex shedding from the HYCOM global model
- · Hurricane activity, generating high currents on local and regional scales
- · Rapidly shallowing bathymetry. with tidal and wind driven currents becoming influential close to the coastline



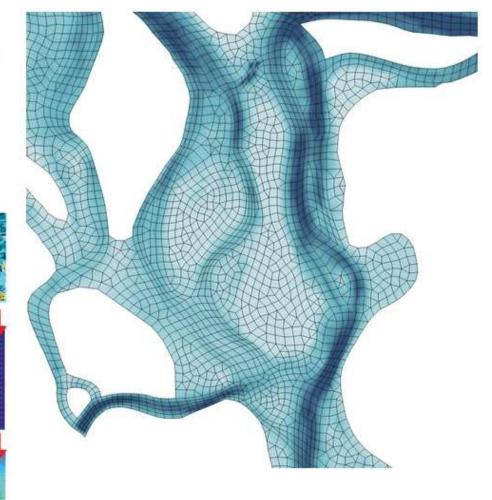
HYCOM (or other) Ocean Circulation



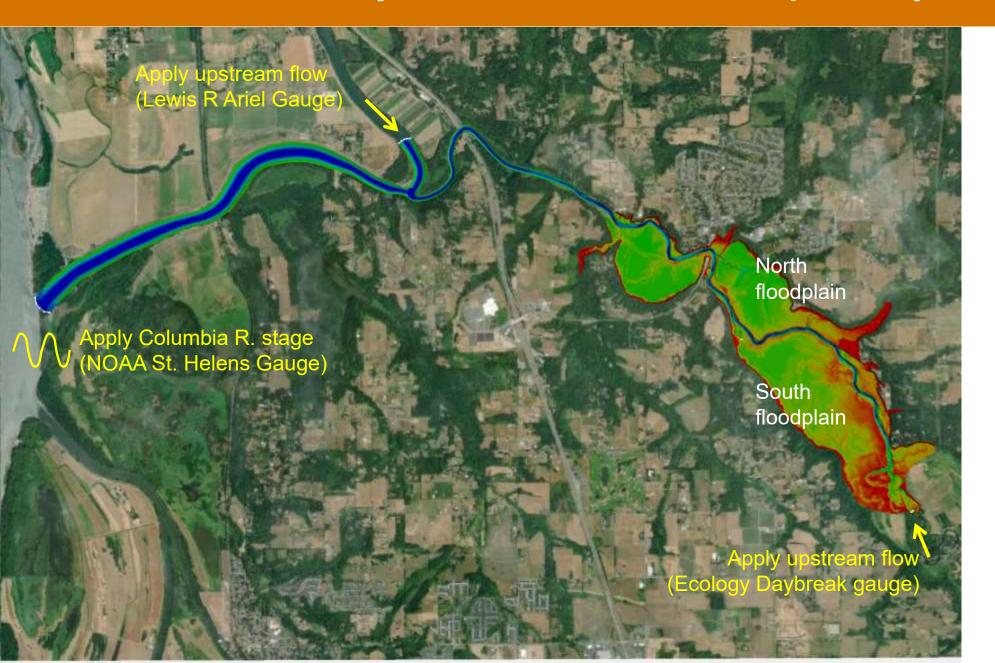
Nesting with advanced 3D boundary transfer (3D + tides + wind)



TUFLOW FV Ocean + winds + local effects



#### East Fork Downstream Hydro Model – La Center Floodplain Projects (2012-205)



Modeled levee breach scenarios at northern and southern floodplain wetland segments upstream of La Center.

Estimated relationship between Daybreak and Heisson gauge data, to allow application of a wider range of flows.



### East Fork Downstream Hydro Model – La Center Floodplain Projects (2012-2015)



Began long term collection of East Fork Lewis River stage data from La Center to above Mason Creek confluence



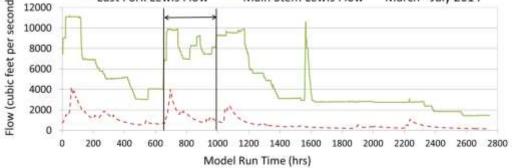
#### East Fork Downstream Hydro Model – La Center Floodplain Projects (2012-2015)

- Initial model validation at 1-year flood return interval and low-flow

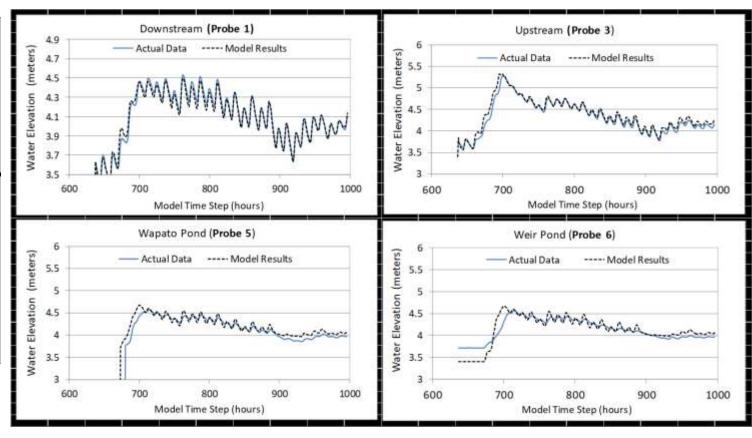
boundary condition inputs:

—Columbia R. Elevation at St. Helens, March - July 2014

Modeled Time Period (stime = 637 - 996 hours)



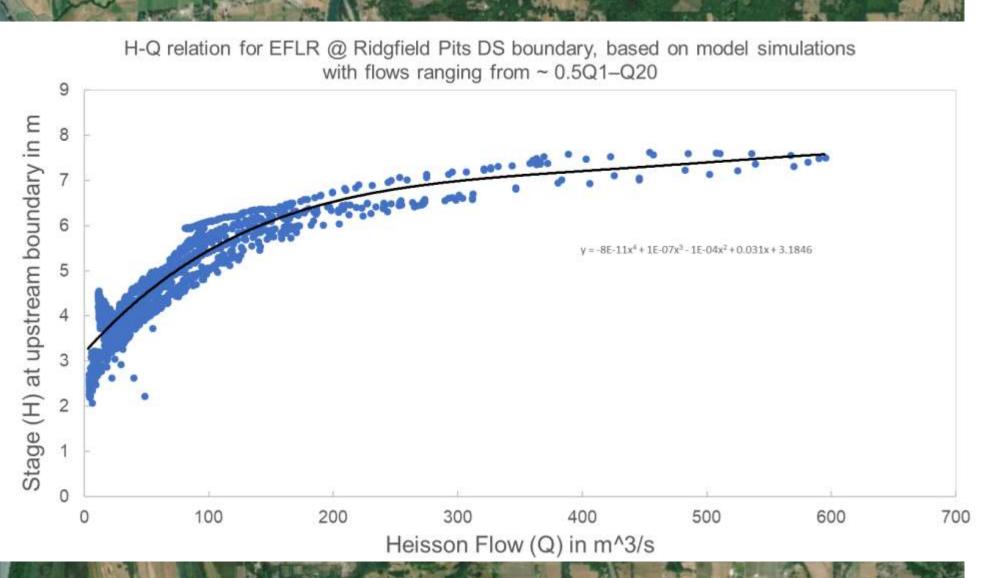
results, simulated vs. observed:



- Later calibrations were performed for 2-year and 80-year (Dec. 2015) flood return intervals



#### East Fork Hydro Model Phase 2 – Add Ridgefield Pits/Daybreak Upstream (2018)



Used Downstream model to establish stage-discharge (H-Q) relationship that can be applied to run the Ridgefield Pits/Daybreak model

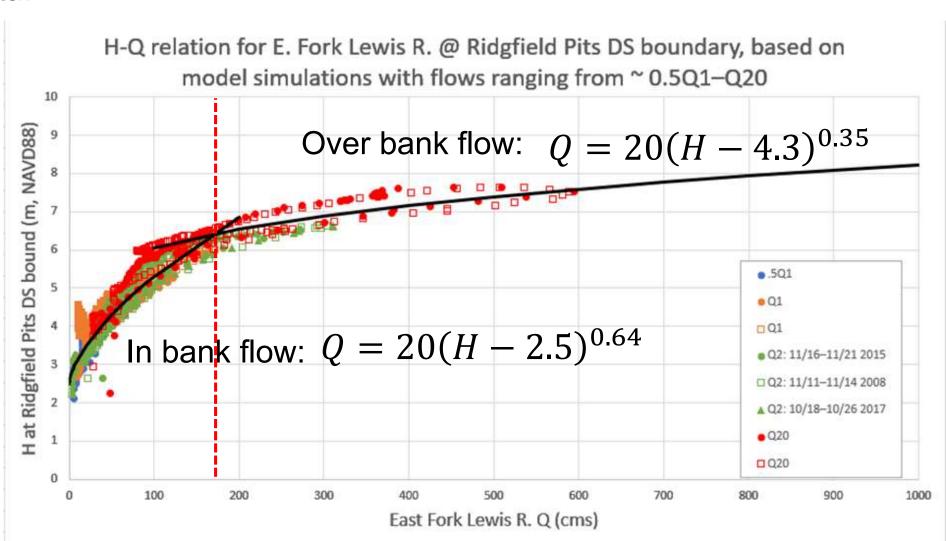


# Model hydraulic inputs: H-Q rating curve

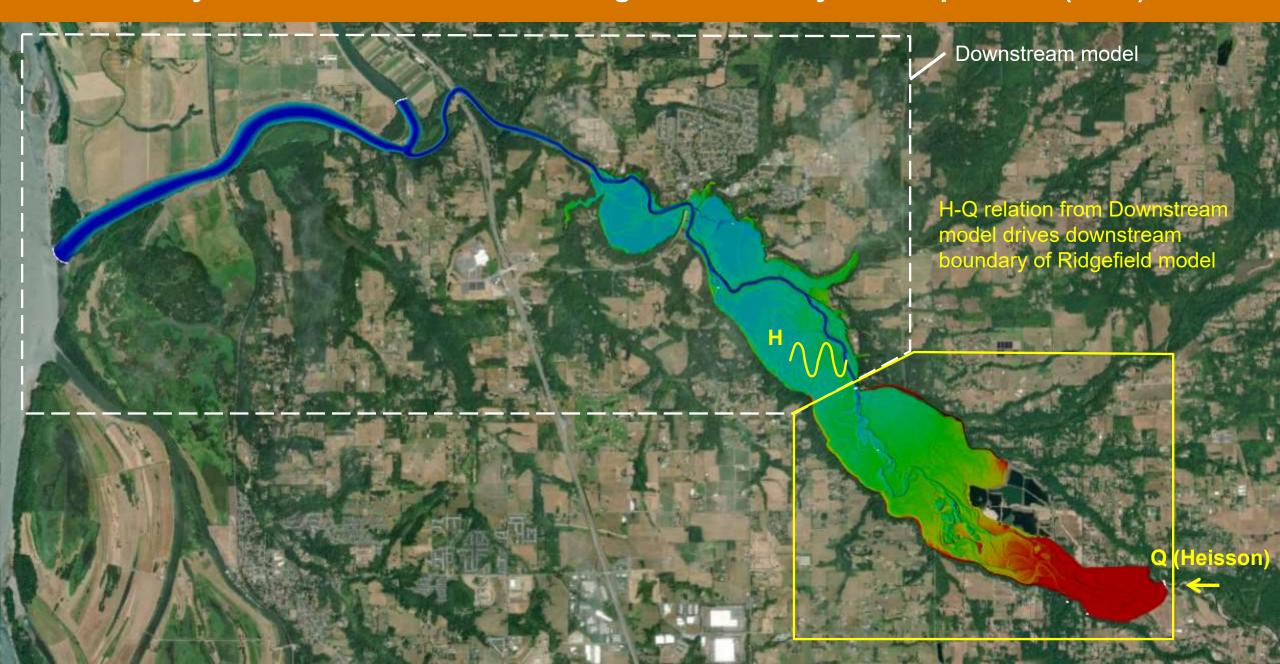
Established relationship between upstream flow (Q) and downstream water level based on available data:

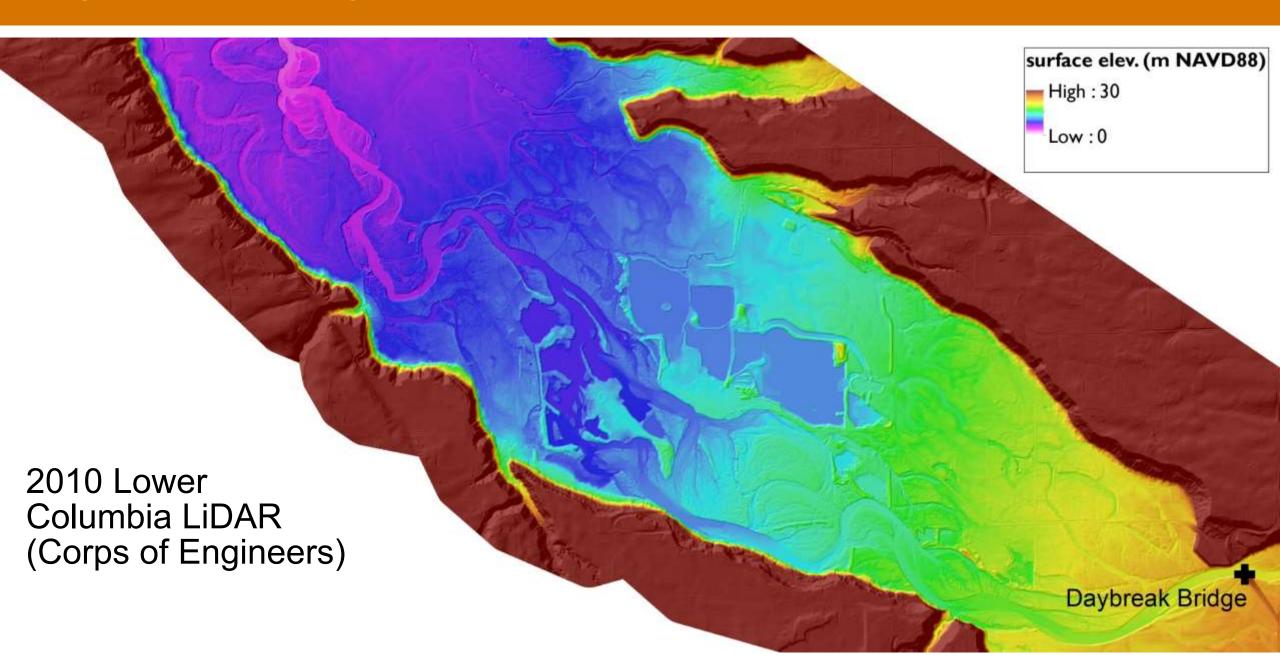
$$Q = C_r (H - a)^{\beta}$$

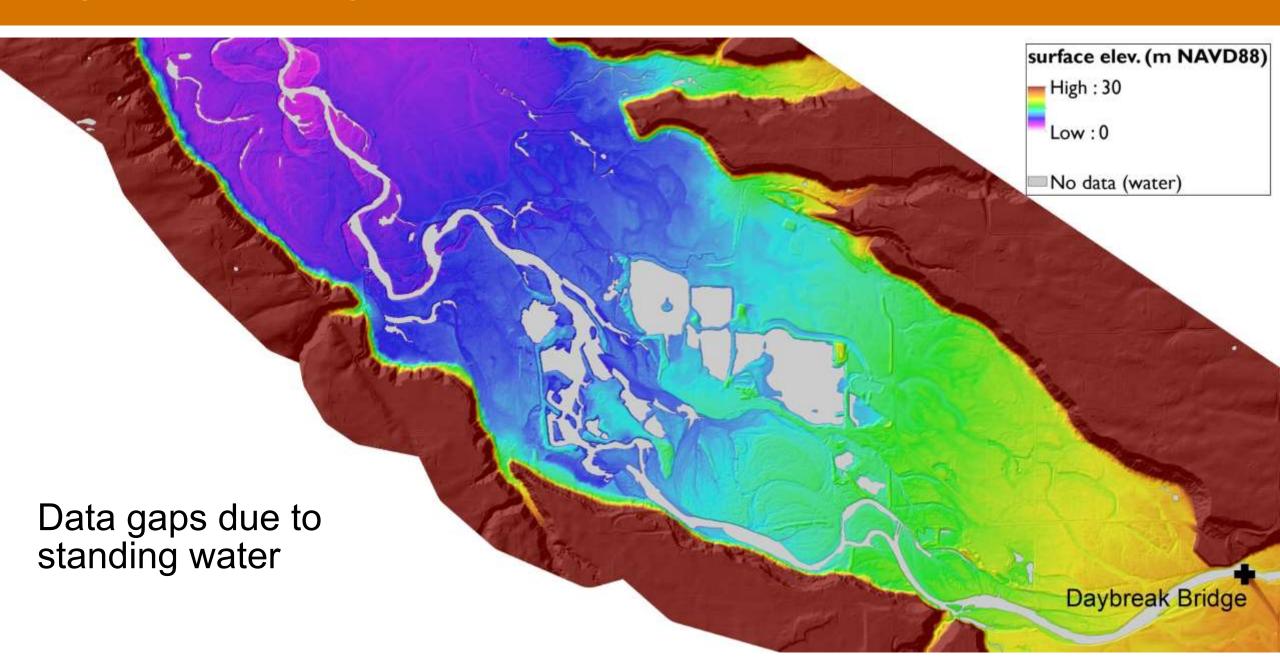
C<sub>r</sub>, β: rating curve constants
a: constant which represents the gauge reading corresponding to zero discharge.

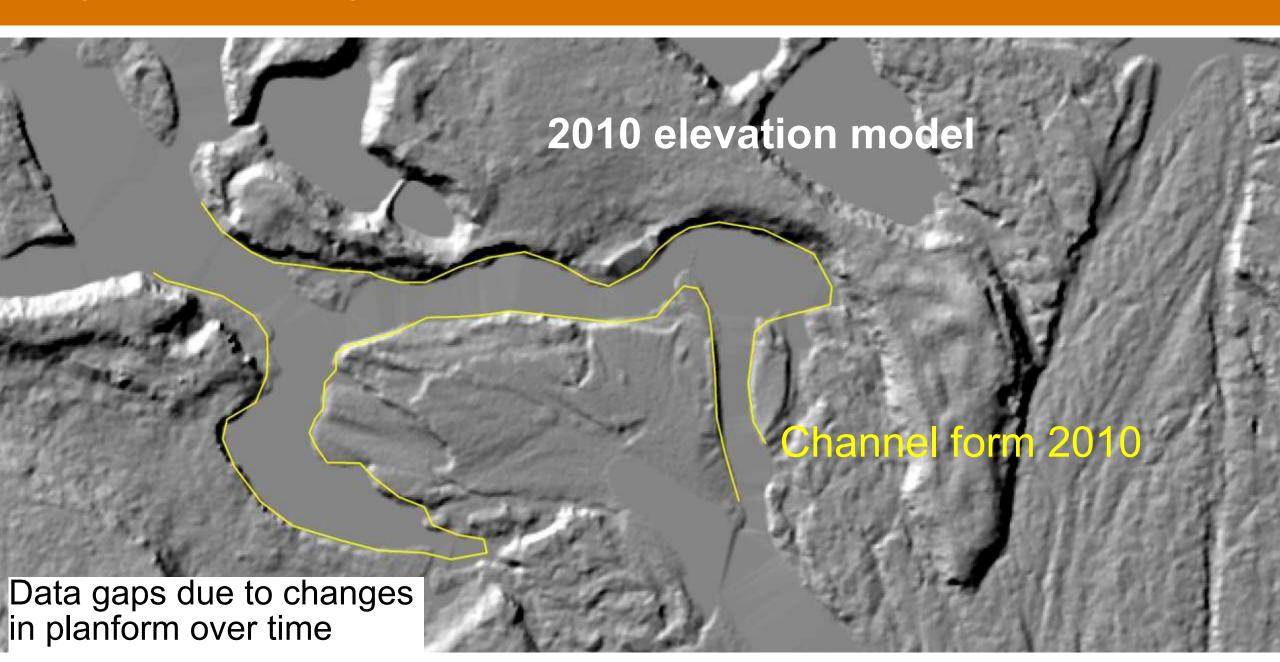


### East Fork Hydro Model Phase 2 – Add Ridgefield Pits/Daybreak Upstream (2018)



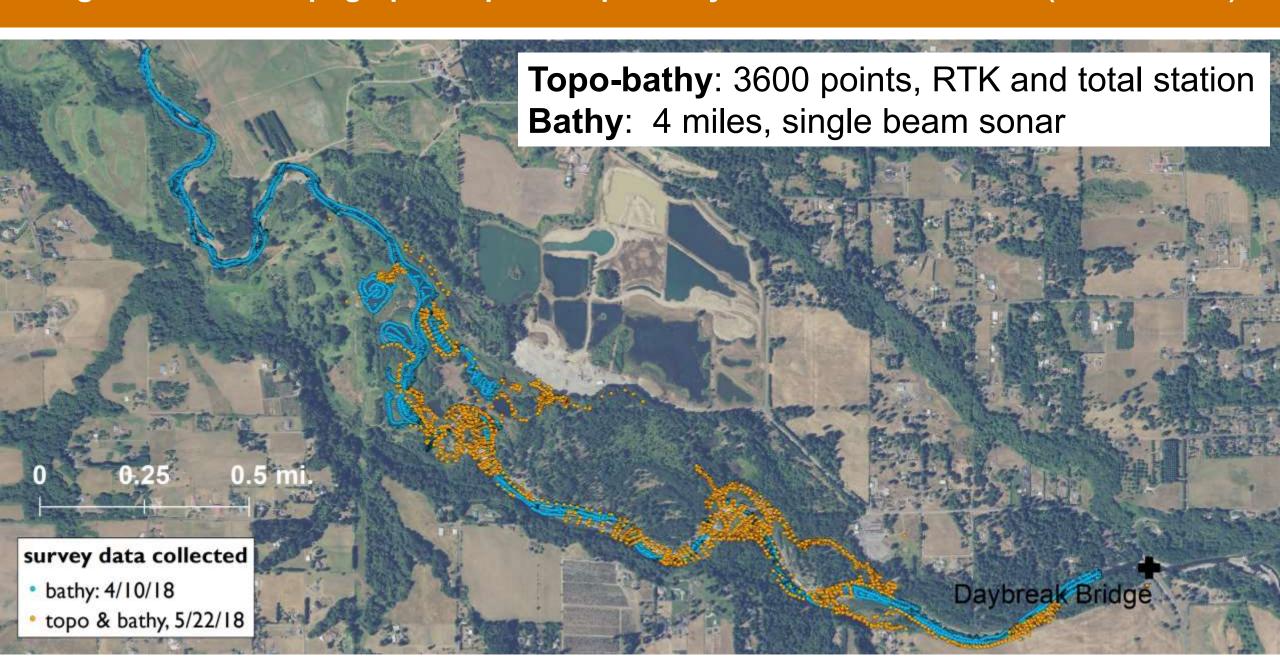




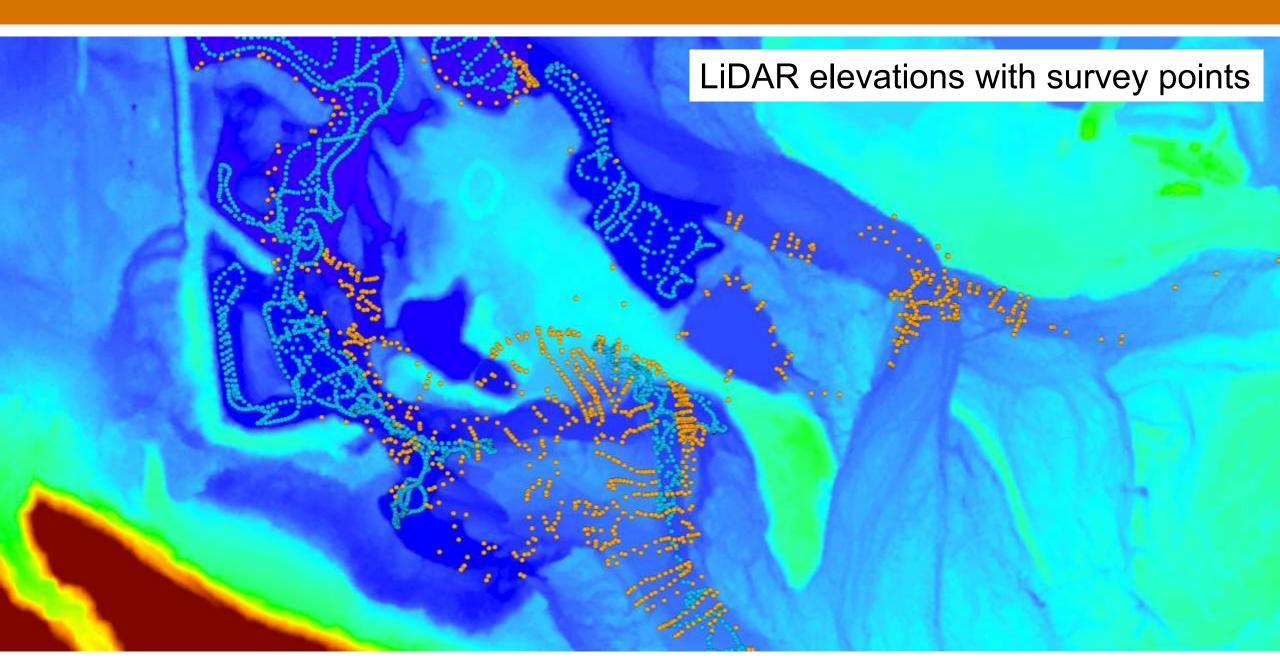




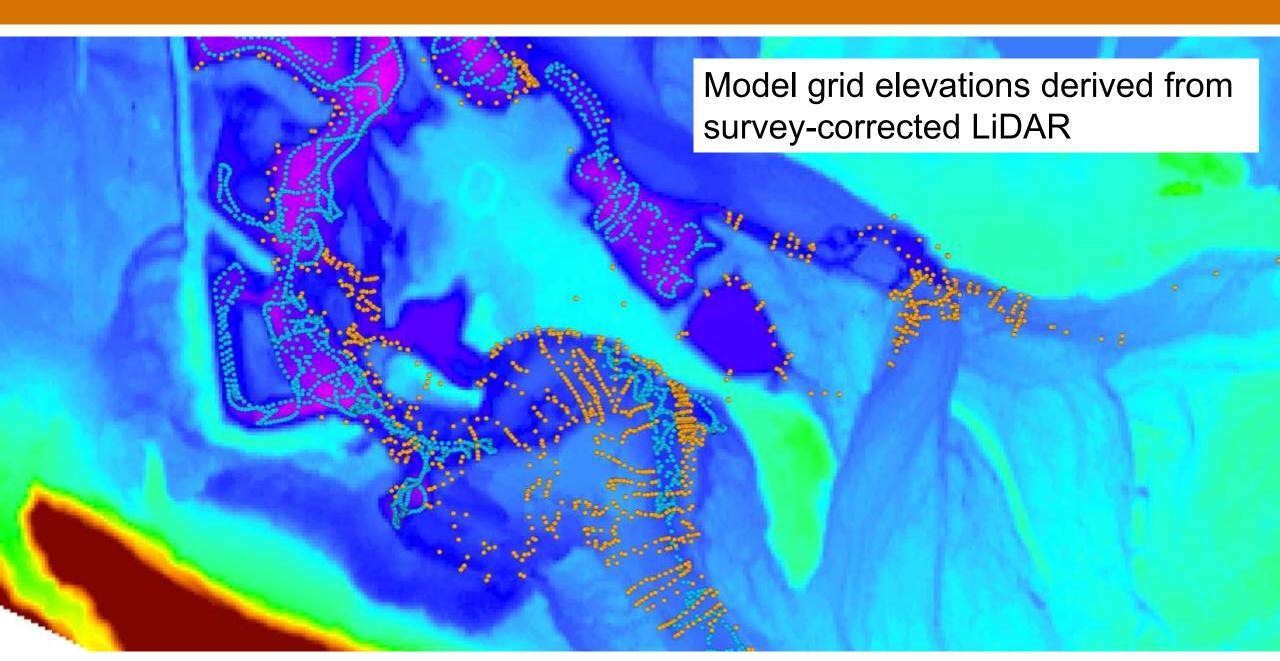
#### Ridgefield Model Topographic Inputs: Topo-bathy data collected in 2018 (I-fluve/LCEP)



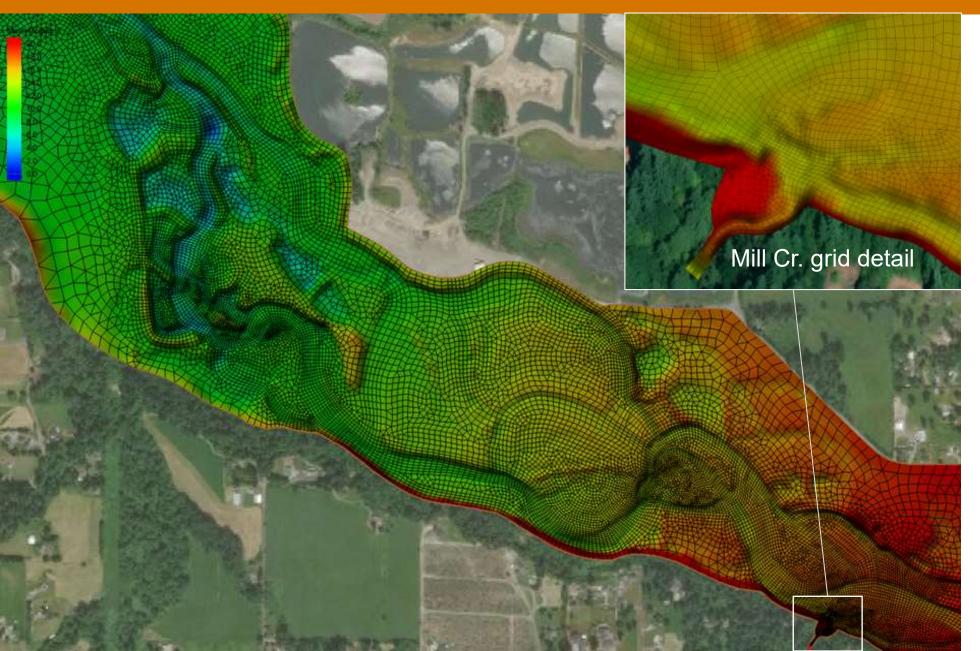
Ridgefield Model Topographic Inputs: Topo-bathy data collected in 2018 (I-fluve/LCEP)



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### Ridgefield Model Grid Development



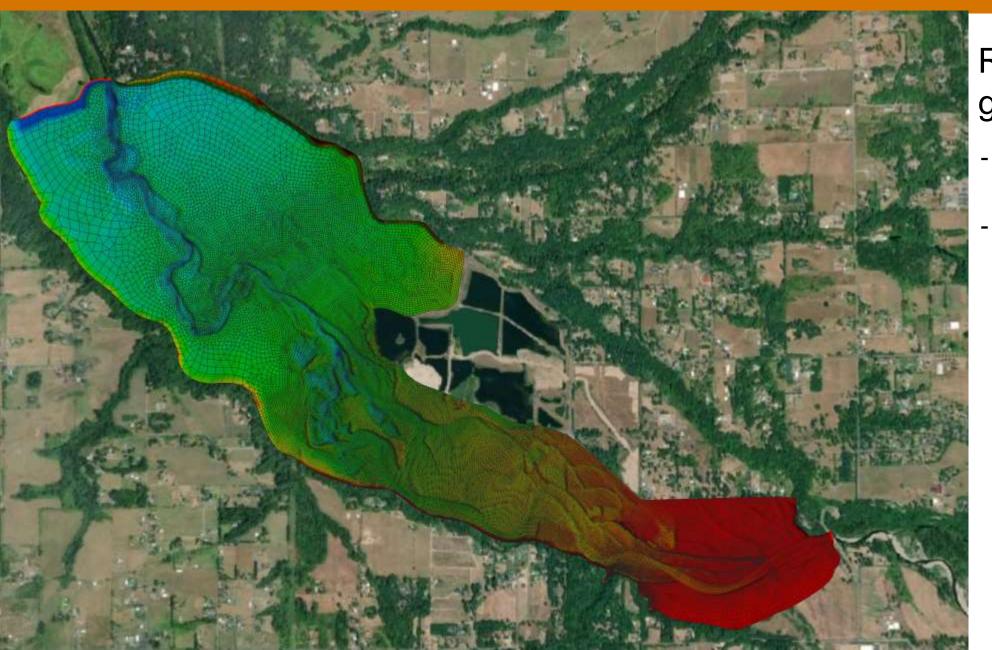
# Initial model grid

- Cells conform to channel geometries (flexible mesh)
- High cell resolution

   (2m) where needed to
   effectively represent
   existing topography.
- Lower cell resolution elsewhere to minimize model run times



### Ridgefield Model Grid Development

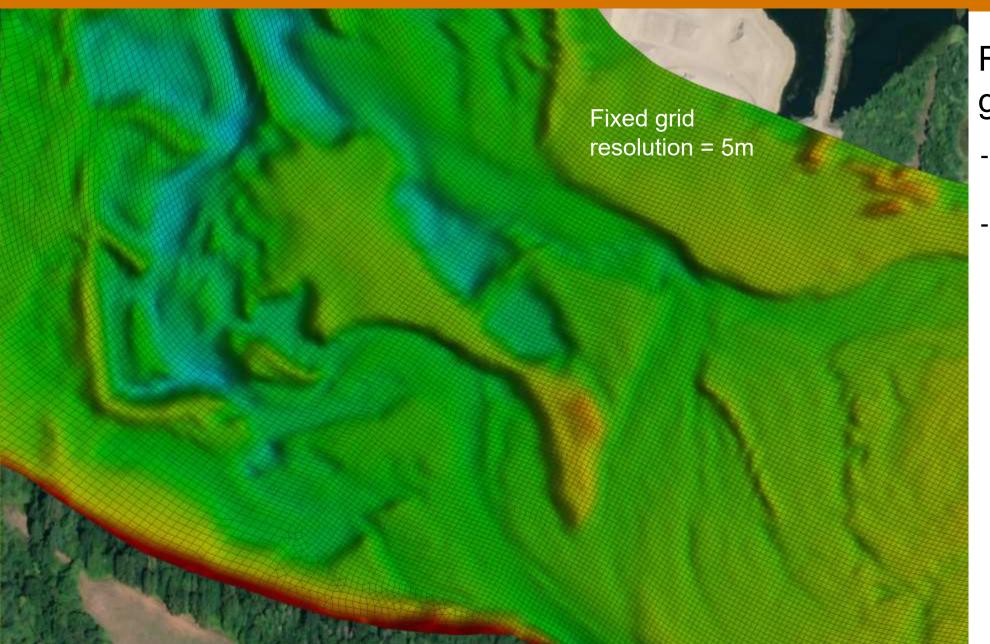


# Revised model grid

- Expanded coverage through DS floodplain
- Switched to semi-fixed grid in project area to:
  - improve performance of sediment/morphodynamic module
  - eliminate grid biaseswhen comparingdesign alternatives



### Ridgefield Model Grid Development

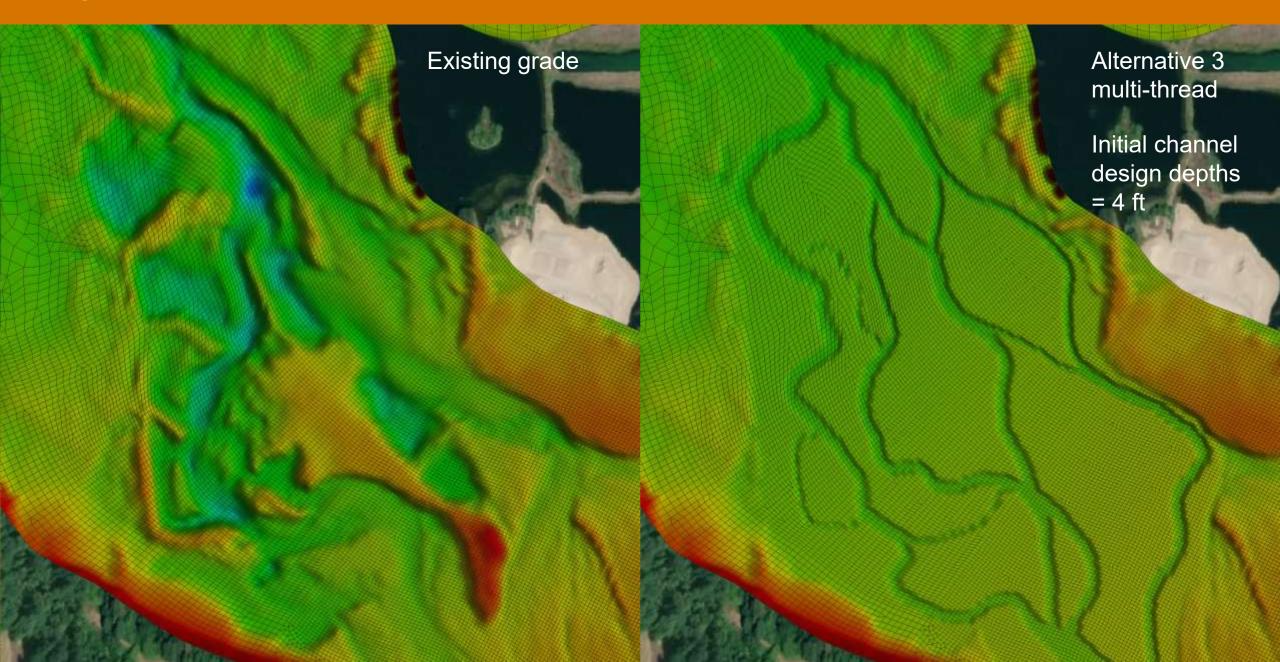


# Revised model grid

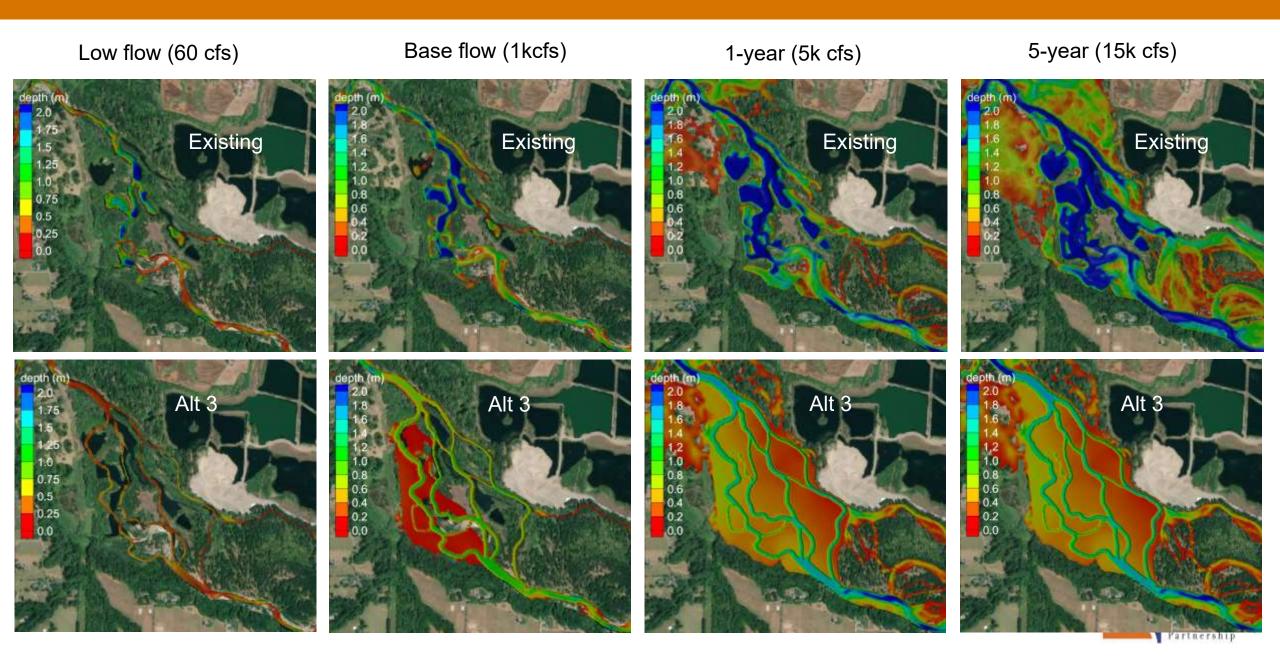
- Expanded coverage through DS floodplain
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  - eliminate grid biaseswhen comparingdesign alternatives



### Ridgefield Model Grid Development – Alternative 3, multi-thread

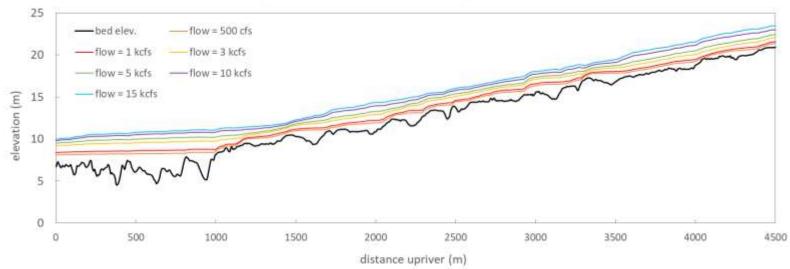


### Ridgefield Model Hydro Results Comparison – Simulated Water Depths

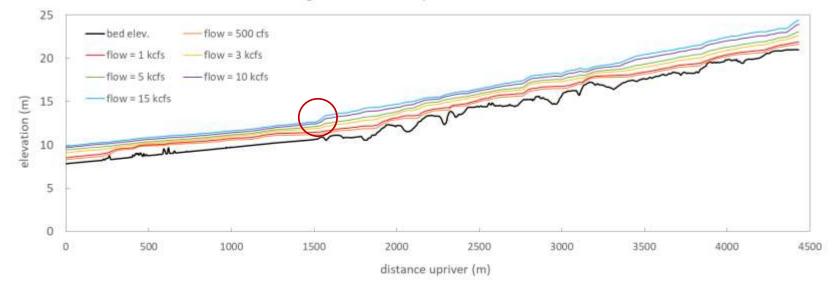


#### Ridgefield Model Hydro Results Comparison – Longitudinal Profiles



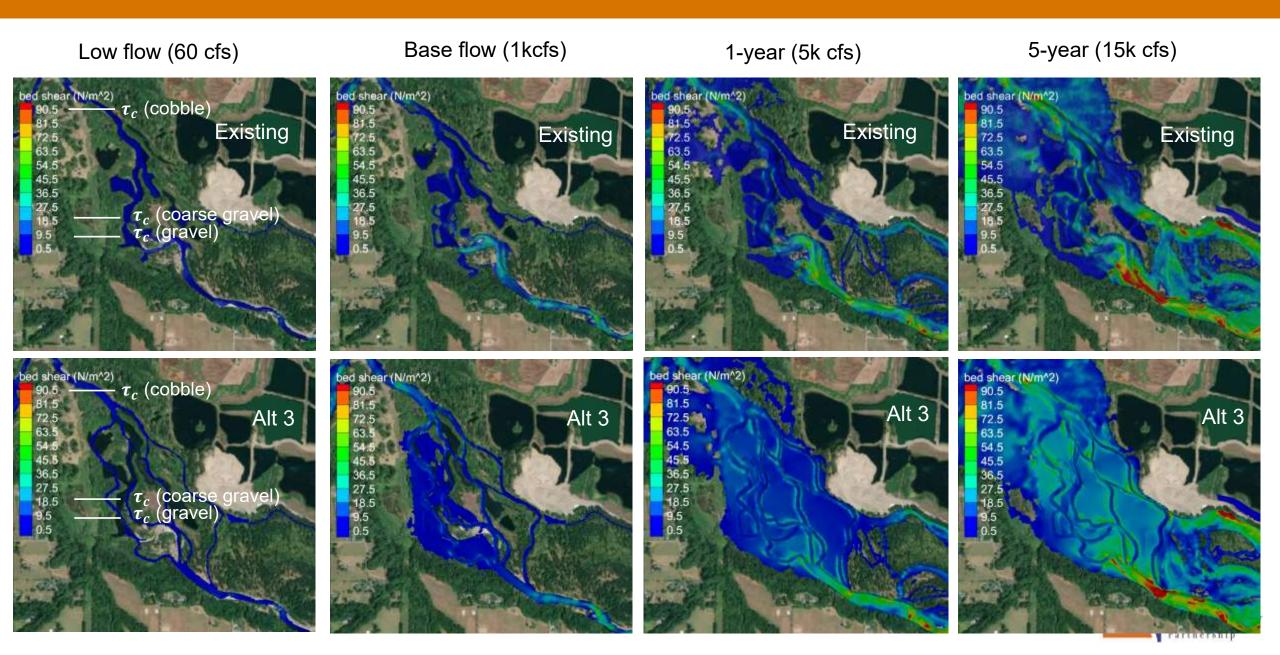


#### longitidutinal surface profiles, Alt3 model





#### Ridgefield Model Hydro Results Comparison – Simulated Bed Shear Stress



#### **Grain Size and Critical Shear Stress**

#### **Critical Shear Stress**

$$\theta_{cr} = Y_{cr} = \frac{0.24}{D_*} + 0.055[1 - \exp(-0.020D_*)]$$
 (23)

$$\theta_{cr} = Y_{cr} = \frac{0.30}{1 + 1.2D_*} + 0.055[1 - \exp(-0.020D_*)]$$
 (24)

Eq. (23) is fitted to the original Shields curve, and (24)  $\theta_{cr}$  contains the modification for small  $D_*$ . The discussers have

with:

D<sub>\*</sub> =  $d_{50}$  [(s-1) g/ $v^2$ ]<sup>1/3</sup> = dimensionless sediment size (m),

 $\theta = \tau_{b,c}/[(\rho_s - \rho_w)gd_{50}] = Shields parameter (-),$ 

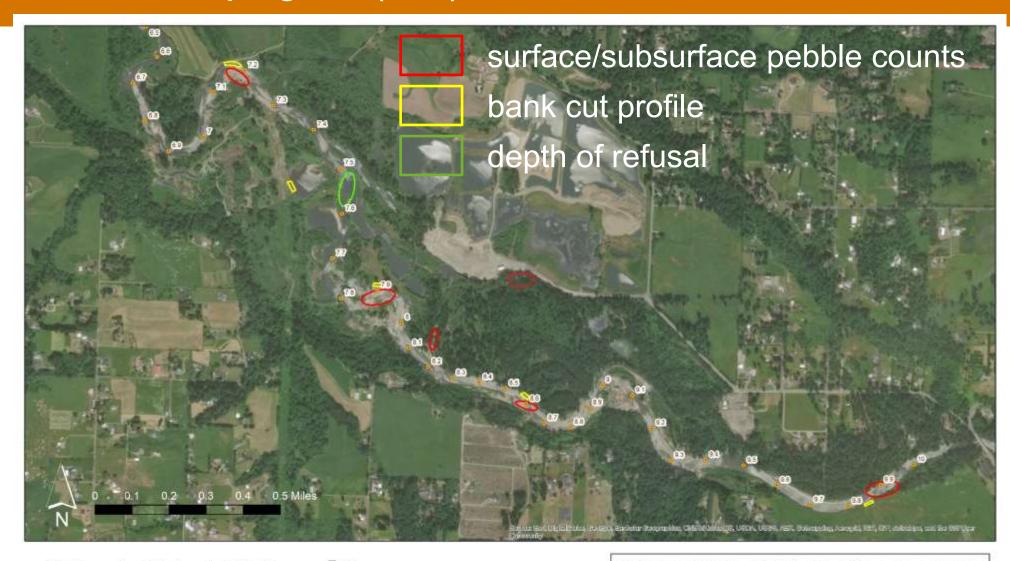
,	Shields	□ Shields (1936)  • Miller, McCave & Komar (1977)  + Luque & Van Beek (1976)  • Mantz (1977)  Δ Hammond & Collins (1979)	Yalin & Karahan (1979)     Katori et al (1984)     Kapdasli & Dyer (1986)     Lee-Young & Sleath (1988)     Kantardgi (1992)
0	0.1	8	

Description	Grain Size (mm)	Tau,c Shields' (1936) (N/m2)	Tau,c Soulsby's (1997) (N/m2)
Fine Sand	1	0.50	0.50
Gravel	10	9.0	9.0
Coarse Gravel	25	22.4	22.4
Cobble	100	89.1	89.1

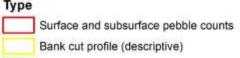
1 Horizontal Bed: Graph of θ<sub>cr</sub> = Y<sub>cr</sub> and D<sub>\*</sub> as Given by Eq. (22) [from Soulsby and White-

Slide source: BWT-WBM (Tuflow FV)

#### **Sediment Sampling Plan (2018)**



EF Lewis Ridgefield Pits Sediment Sampling Plan Created Oct 2018

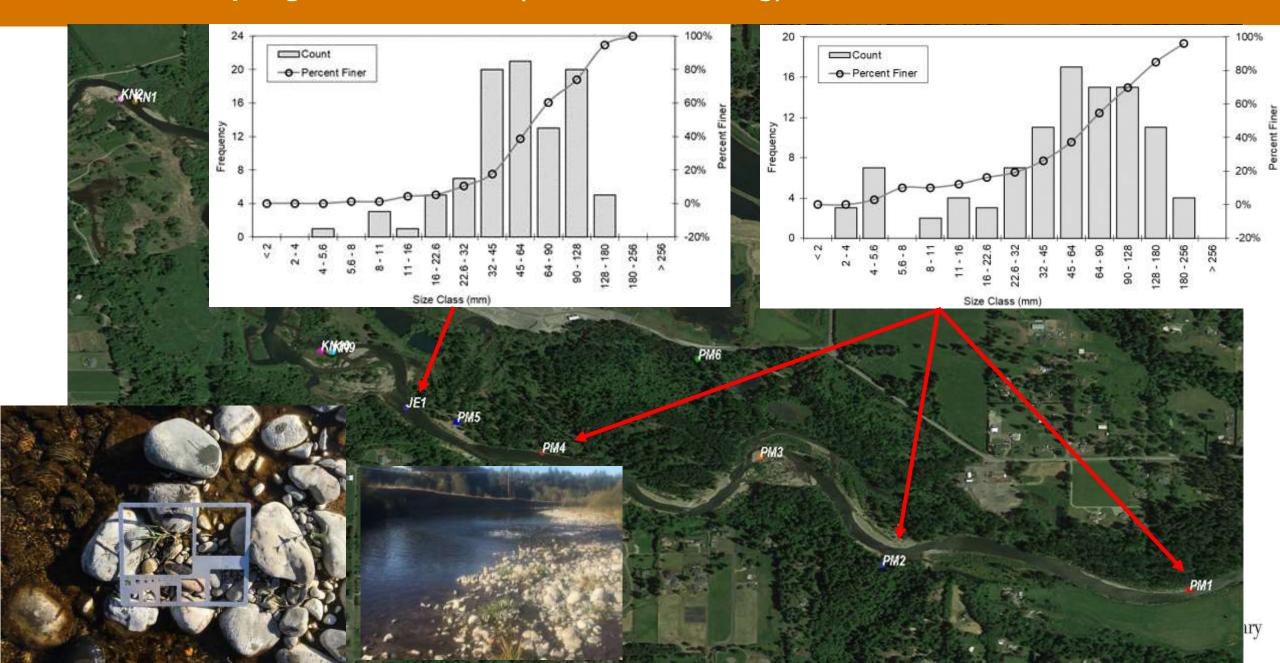


Surface characterization and depth of refusal

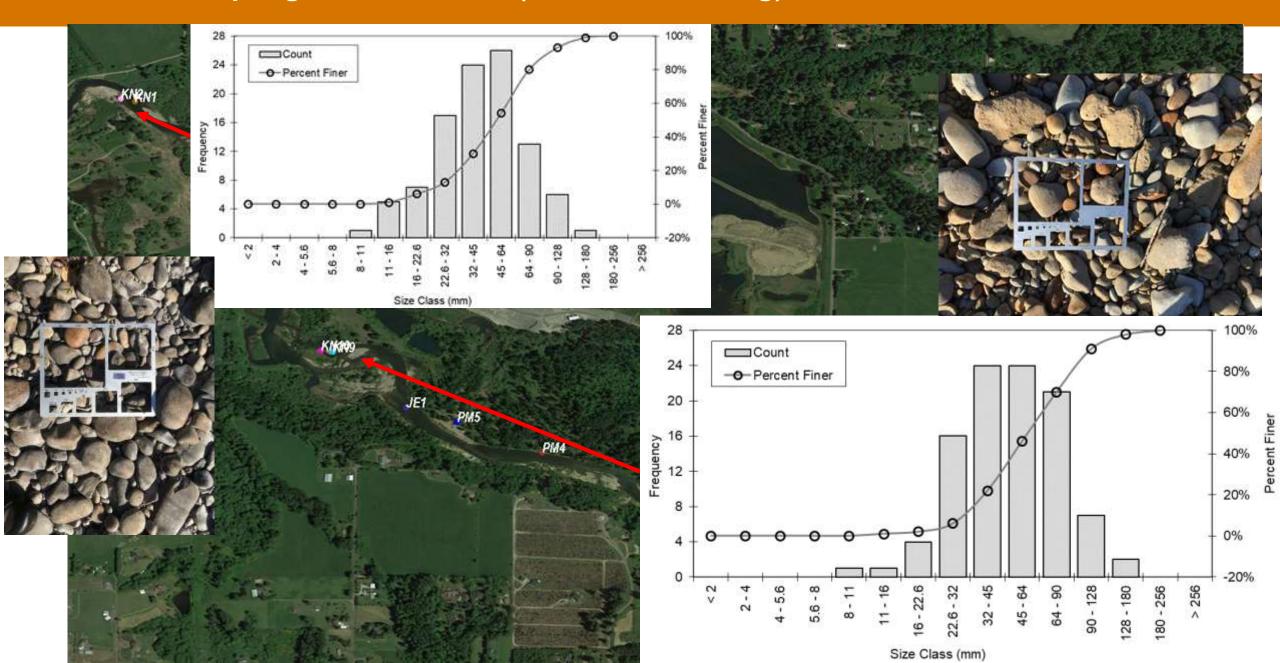
Surface samples to be collected using pebble counts on transects across the active channel. Subsurface counts to be done by removing the surface layer at a representative sampling location and performing pebble counts on the subsurface material as described in Buffington (1996) as cited and described in Bunte and Abt (2001).



### **Sediment Sampling – Bed Material (Surface/Armoring)**



### **Sediment Sampling – Bed Material (Surface/Armoring)**



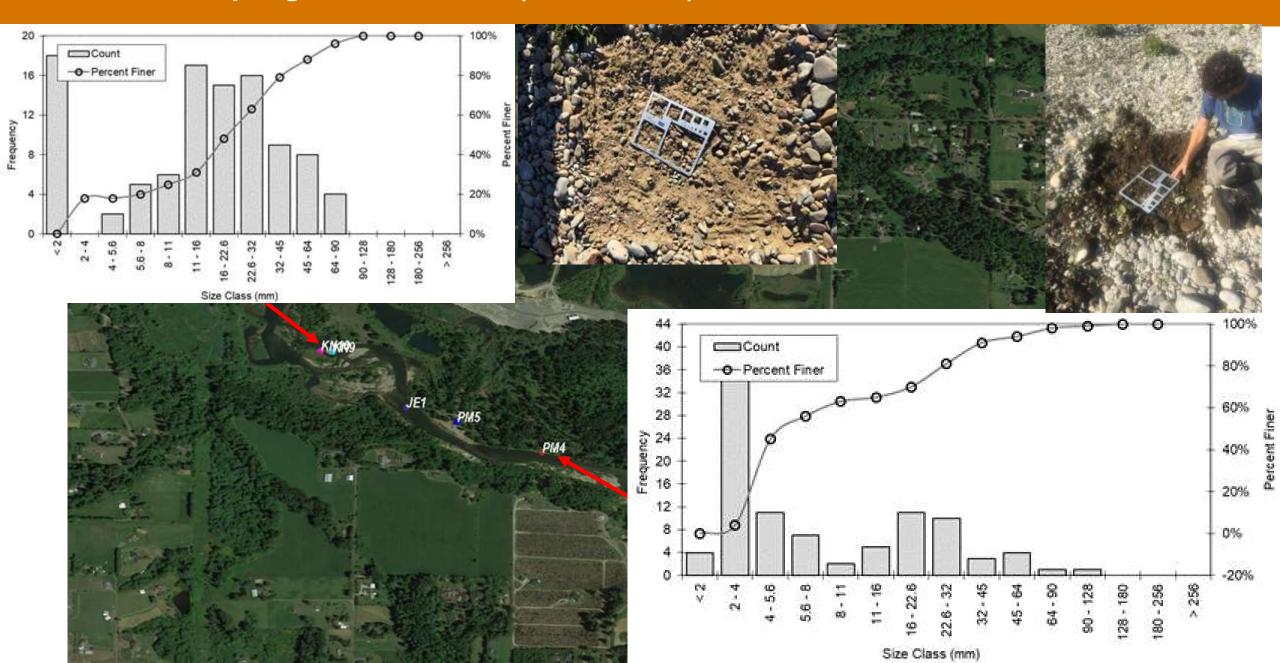
#### **Sediment Sampling – Bed Material (Surface/Armoring)**

- Relatively consistent size distribution throughout the study area
- D10 ≈ 20mm Coarse Gravel
- D50 ≈ 50mm Very Coarse Gravel
- D84 ≈ 100mm Cobble





### **Sediment Sampling – Bed Material (Subsurface)**

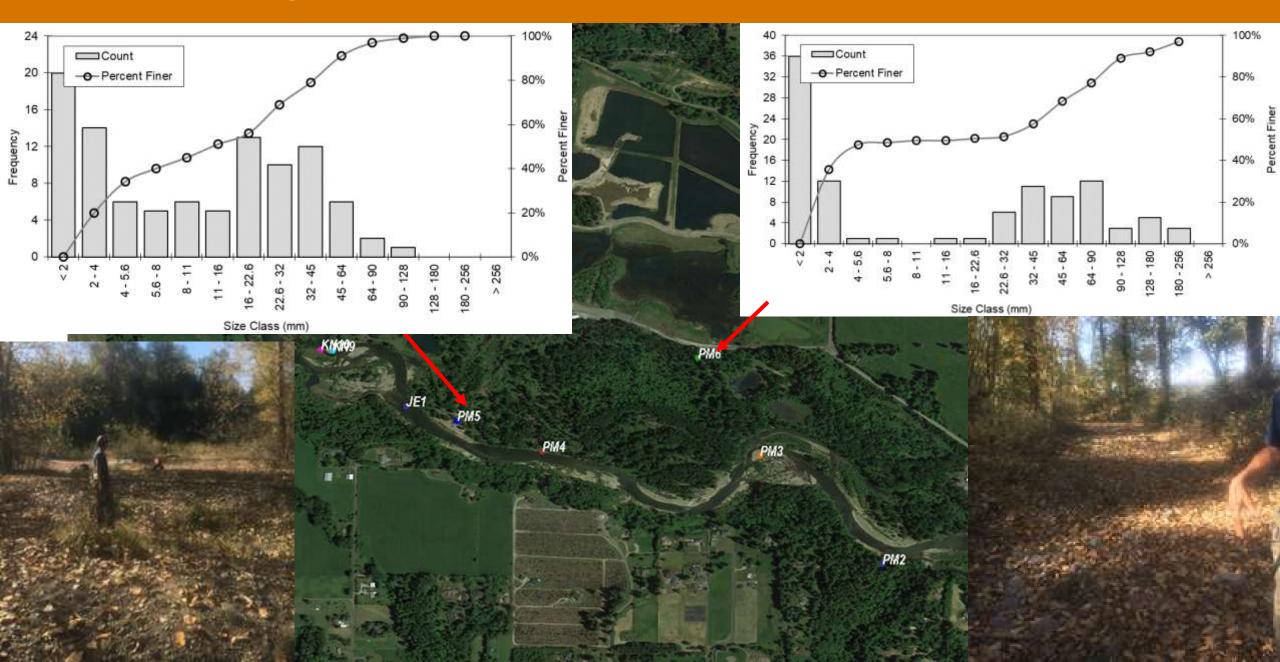


### **Sediment Sampling – Bed Material (Subsurface)**

- Two peaks in grain size, i.e. mixture of:
  - 1. Coarse components: Gravel/Cobble, ~ same as river bed
  - 2. Fine components: Sands, even silts (2mm and less)



### **Sediment Sampling – Floodplain Material**

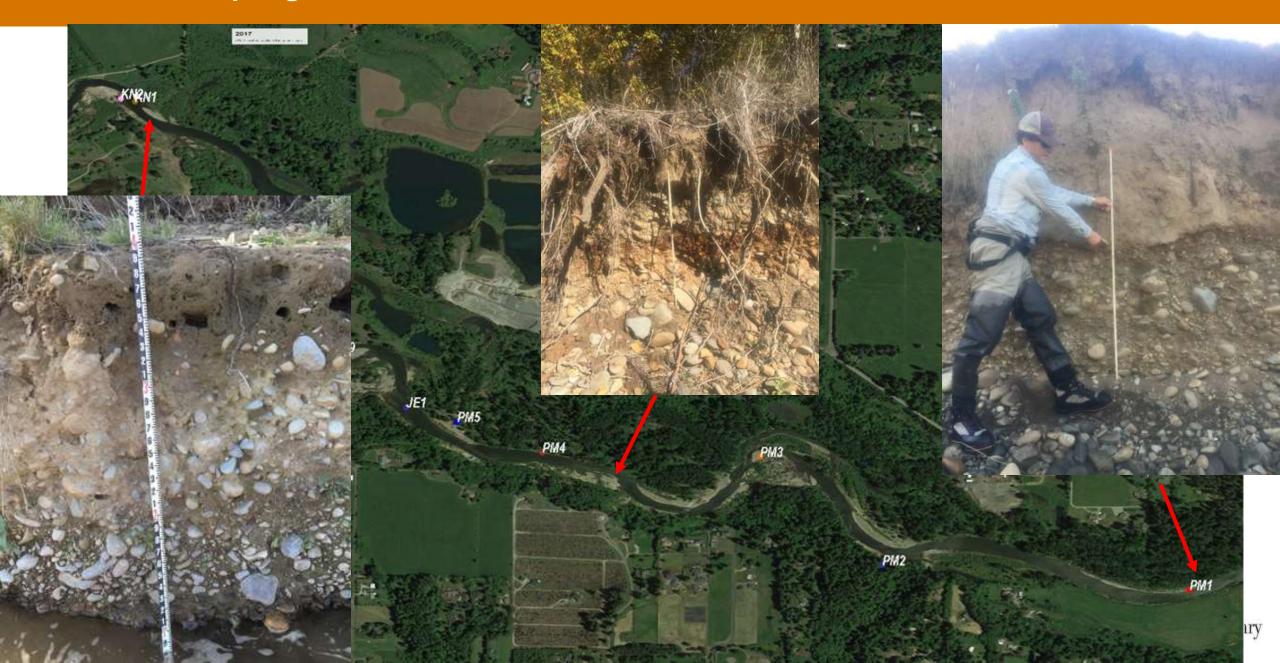


### Sediment Sampling – Floodplain Material

 Also has two peaks in grain size. Similar profiles to riverbed subsurface.



# **Sediment Sampling – Vertical Bank Profile**



# **Sediment Sampling – Vertical Bank Profile**



# **Sediment Sampling – Vertical Bank Profile**

**Type 1:** Single layer of mixed sand, gravel, cobble. Forested floodplains.



**Type 2**: Two layer structure with thick sand/silt layer over the Type 1 layer. More dominant.



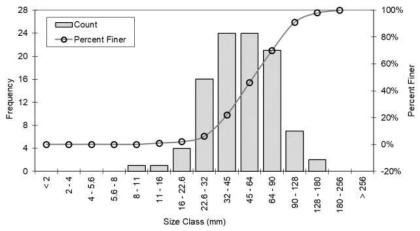
# **Proposed Sediment Parameters for Modeling**

### **Sediment Type A:**

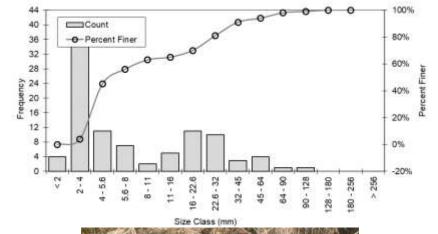
bed surface layer

# **Sediment Type B:**

bed sublayer, forested floodplain



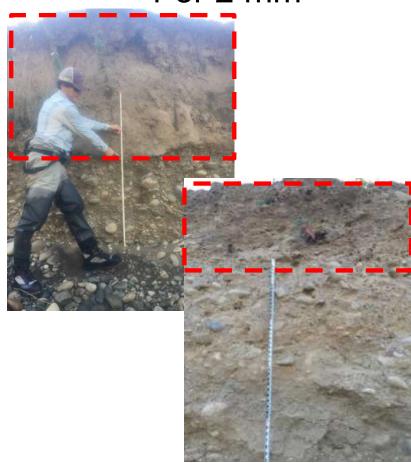






## **Sediment Type C:**

Bank top layer, uniform size of 1 or 2 mm

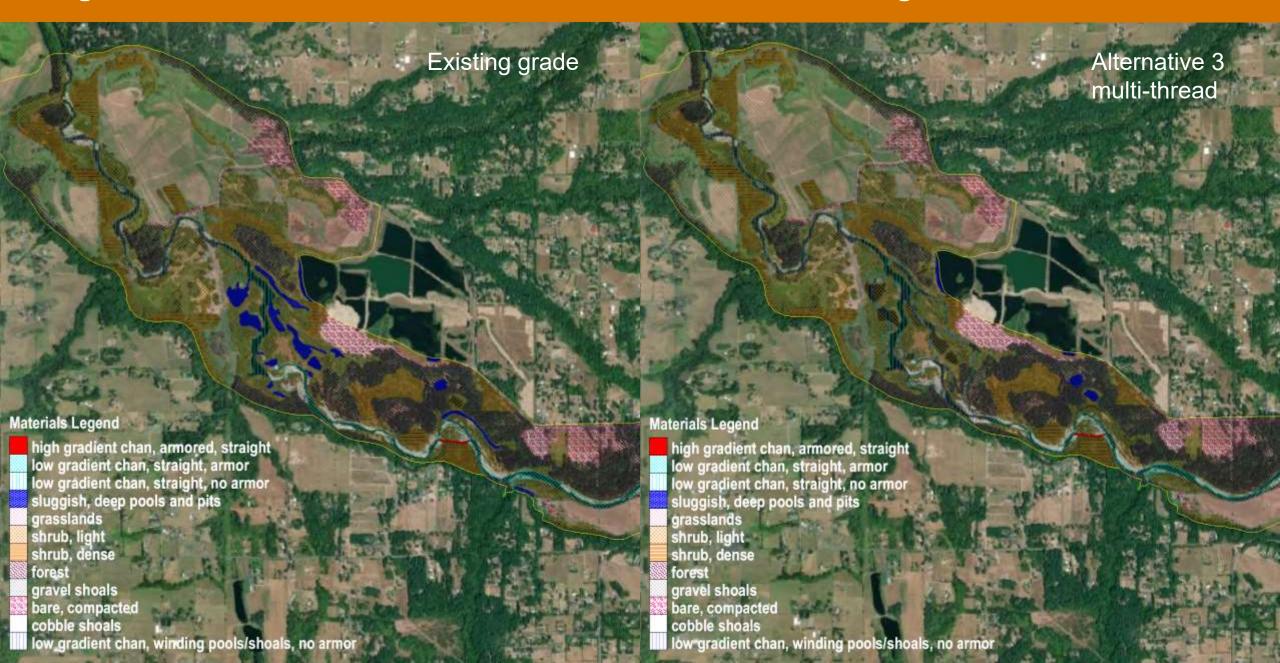


# **Proposed Sediment Parameters for Modeling**

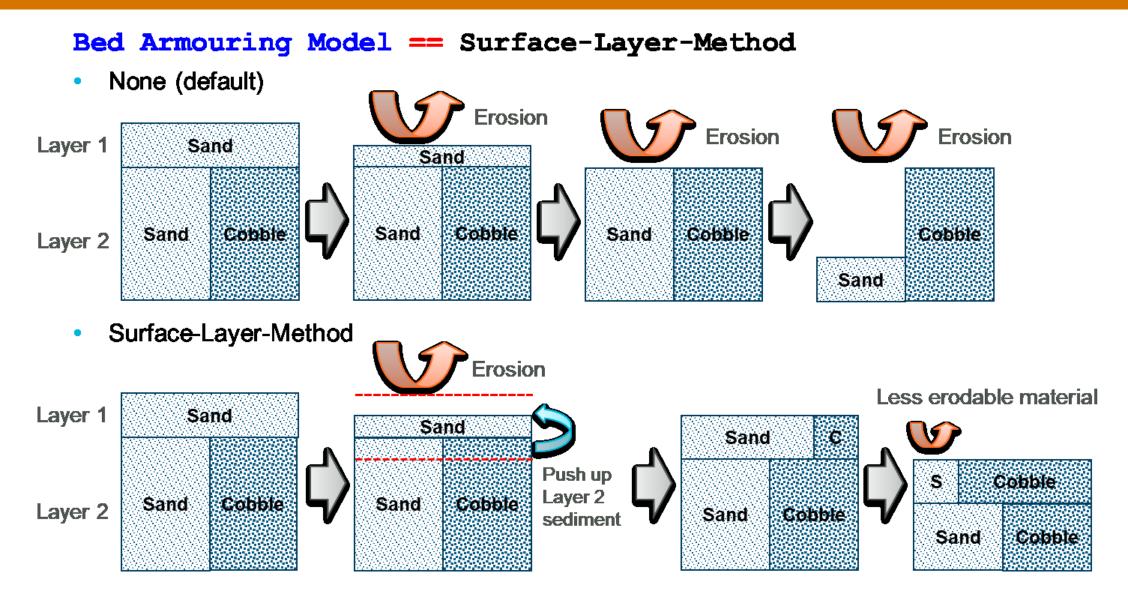
Area	Layer	Sediment Type*	Grain Size			
			2mm Sand	20mm Gravel**	50mm Gravel	100mm Cobble
River Bed	1	Α		$\sqrt{}$	$\sqrt{}$	$\sqrt{}$
	2	В	$\checkmark$	$\sqrt{}$	$\checkmark$	$\sqrt{}$
River Bank	1	С	$\sqrt{}$			
	2	В	$\checkmark$	$\sqrt{}$	$\checkmark$	$\sqrt{}$
Forestry Floodplain	1	В				



#### Ridgefield Model Grid – Friction and Sediment Parameter Assignment



### **Tuflow FV Sediment Module: Armouring Feature**



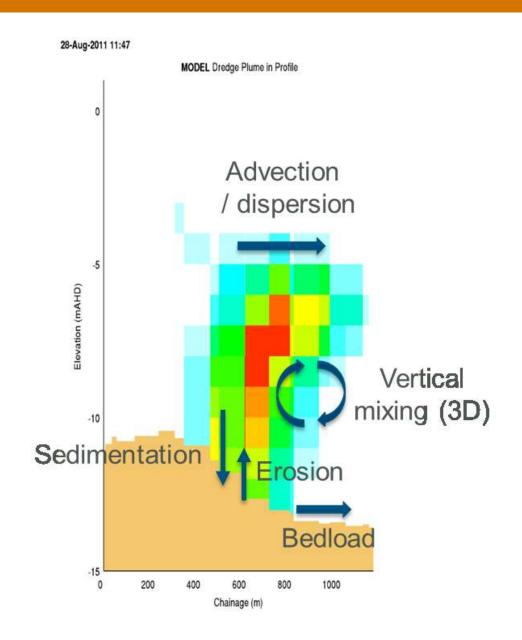
Slide source: BWT-WBM (Tuflow FV)

### **Tuflow FV Sediment/Morpho-dynamic Model Key Features**

#### Suspended load

- Hydraulic module→ advection / dispersion
- Sedimentation / Erosion (Mehta Model)
- Bedload (Meyer-Peter Mueller Model, Wilcock Crowe just added)
- Morphology (adjusts bed elevation in response to sediment transport)
- Multiple sediment fraction
- Multiple bed layer
- Armouring
- 2D/3D

Slide source: BWT-WBM (Tuflow FV)





# **Transport Mode Governing Equations**

#### Suspended Load Erosion Model == Mehta

Mehta's erosion model

$$E = E_r \big( \tau_b / \tau_{b,c} - 1 \big)^{\alpha}$$

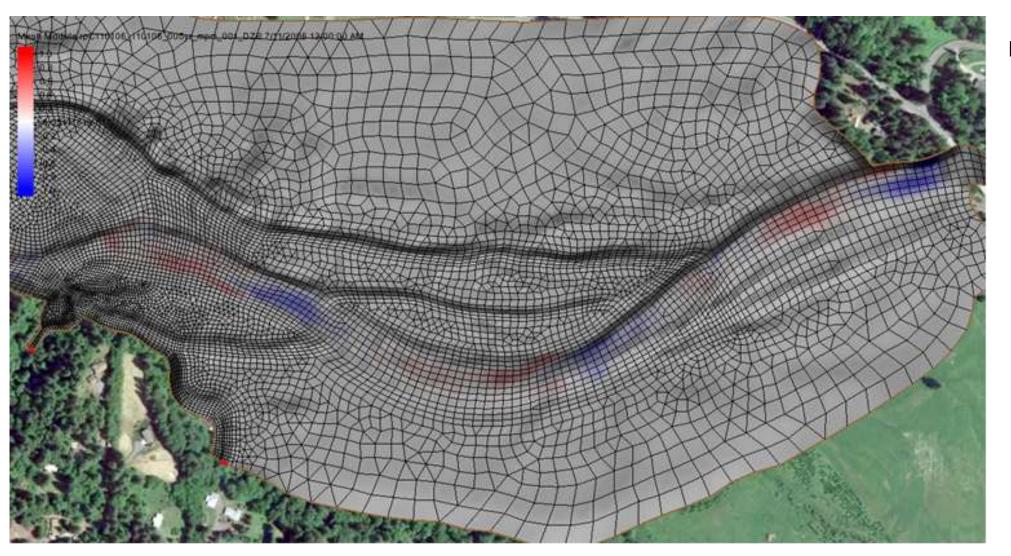
#### Bedload Model == MPM

Meyer-Peter Müller's equation

$$\frac{q_{bv}}{\sqrt{(G-1)gd_{50}^3}} = 8(\tau_{b*} - 0.047)^{3/2}$$

- Condition for sediment transport: Bed shear stress  $(\tau_b)$  > critical shear stress  $(\tau_c)$
- Look at  $\tau_b$  values output by hydrodynamic (or sediment) model to get a sense of where sediment is predicted to move under different flow conditions.
- Compare to known sources (imagery) to see if model is making sense

### Morpho-dynamic Module Results: Bed Elevation Change, post 5-yr event



Bed elevation change (m)







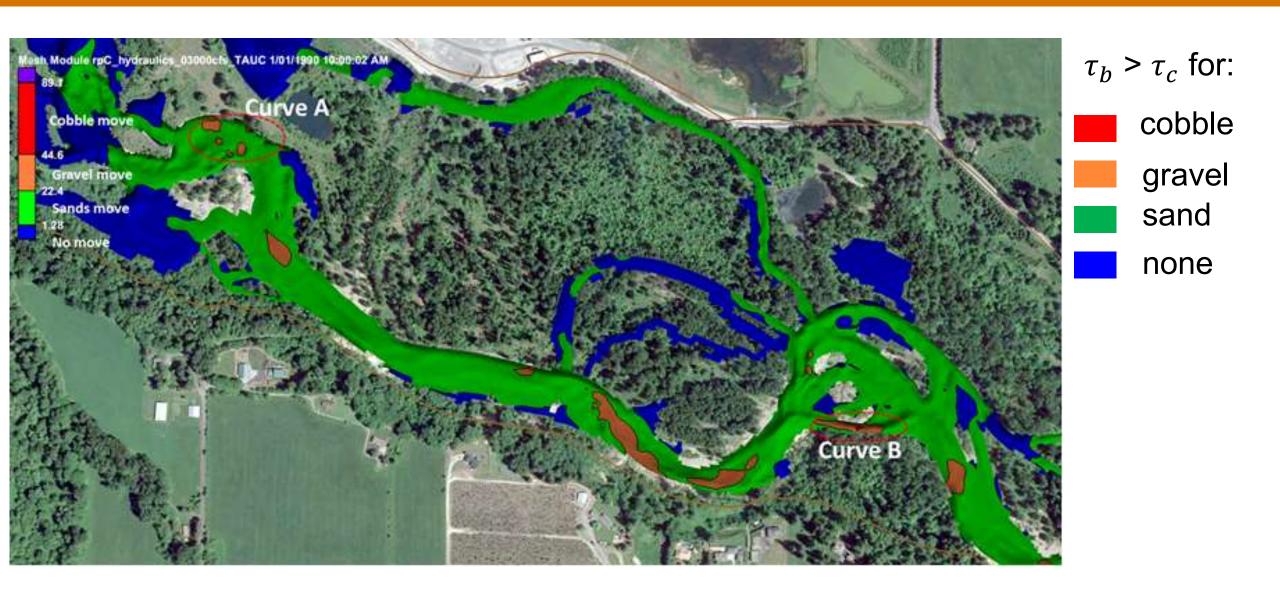
\*\*morpho-dynamic simulations take a long time, so we are waiting to have the restoration alternatives more developed before running additional scenarios



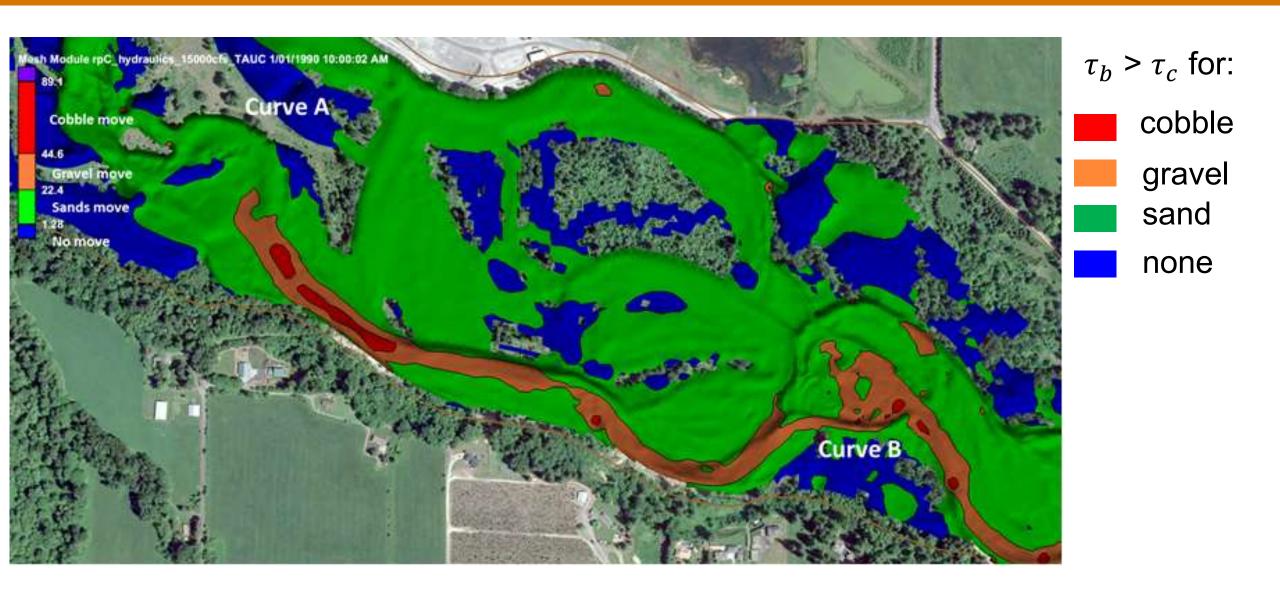
### Model Results: Bed Shear Stress, Q = 1000 cfs (base flow)



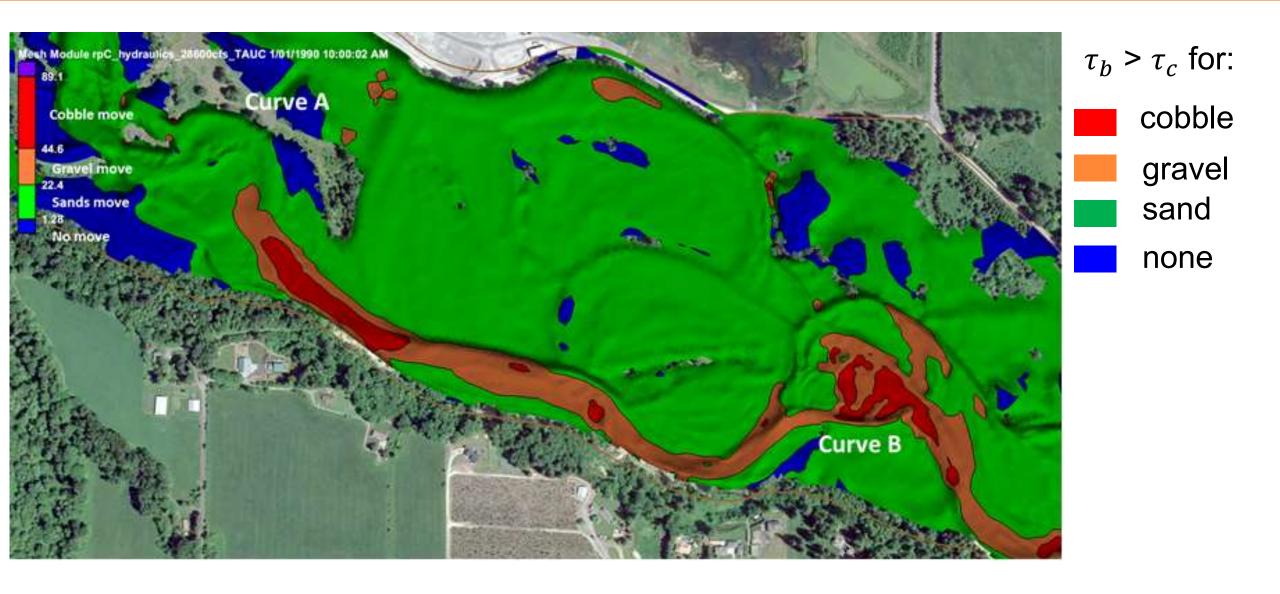
# Model Results: Bed Shear Stress, Q = 3000 cfs (~Q1)



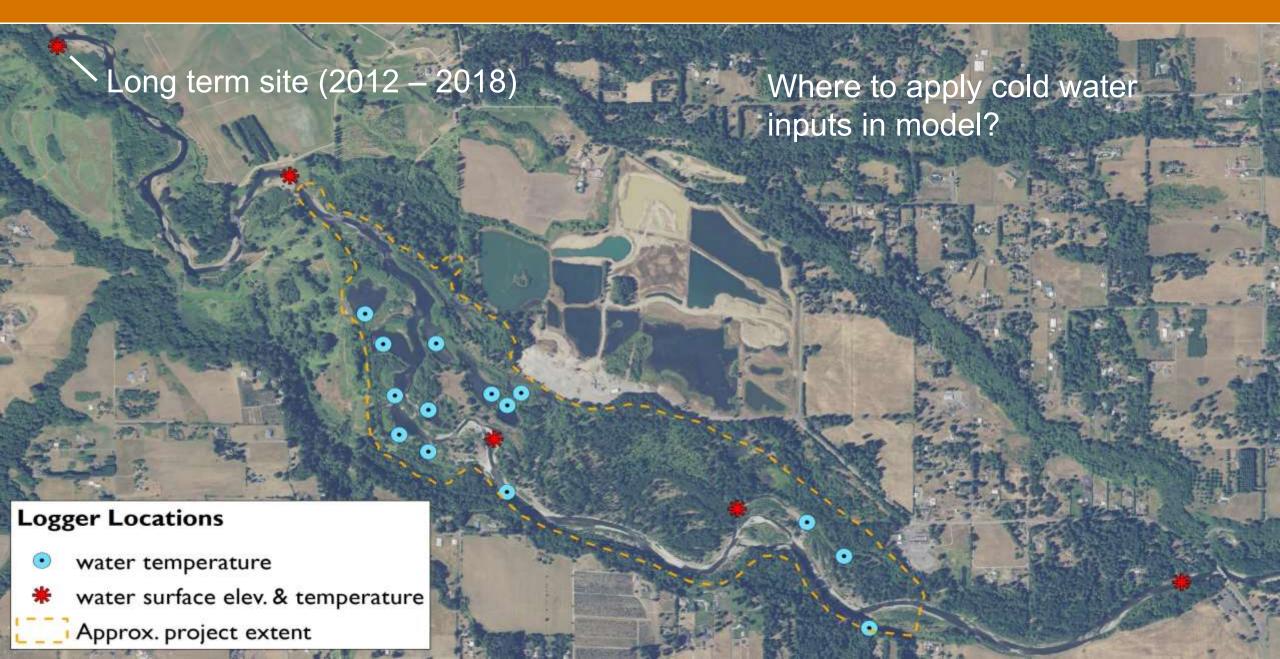
# Model Results: Bed Shear Stress, Q = 15,000 cfs (~Q20)



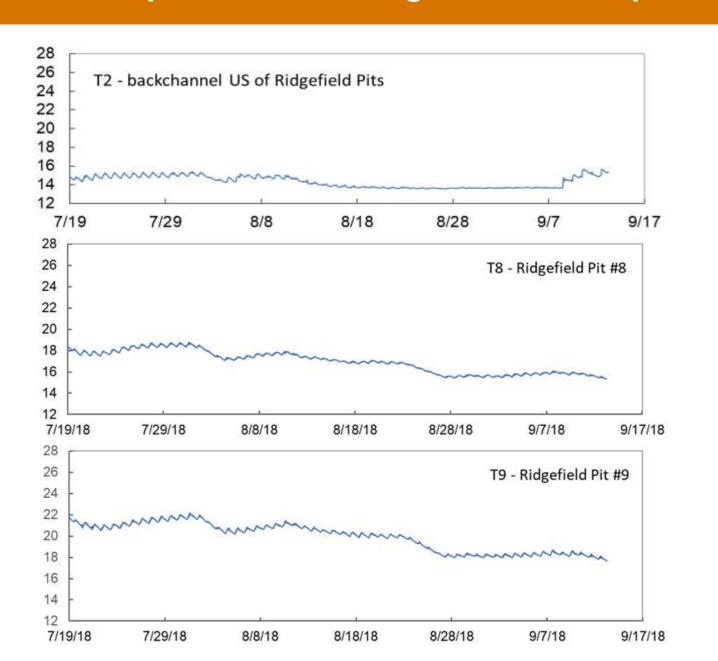
## Model Results: Bed Shear Stress, Q = 28,600 cfs (~Q100)

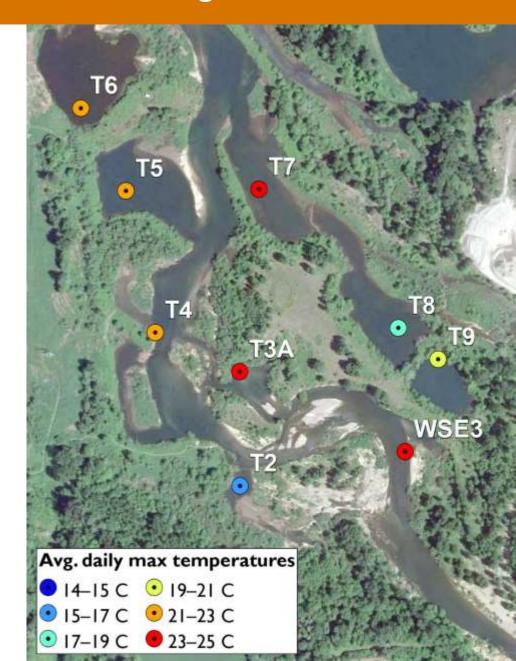


### Water Temperature Modeling – Field Data Plan For Model Input

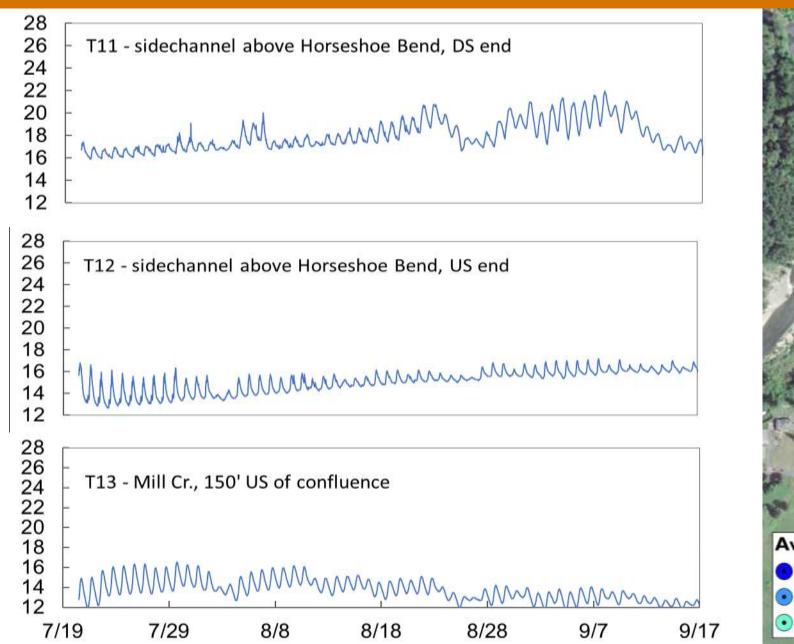


### Water Temperature Modeling - In-situ Temperature Monitoring Results



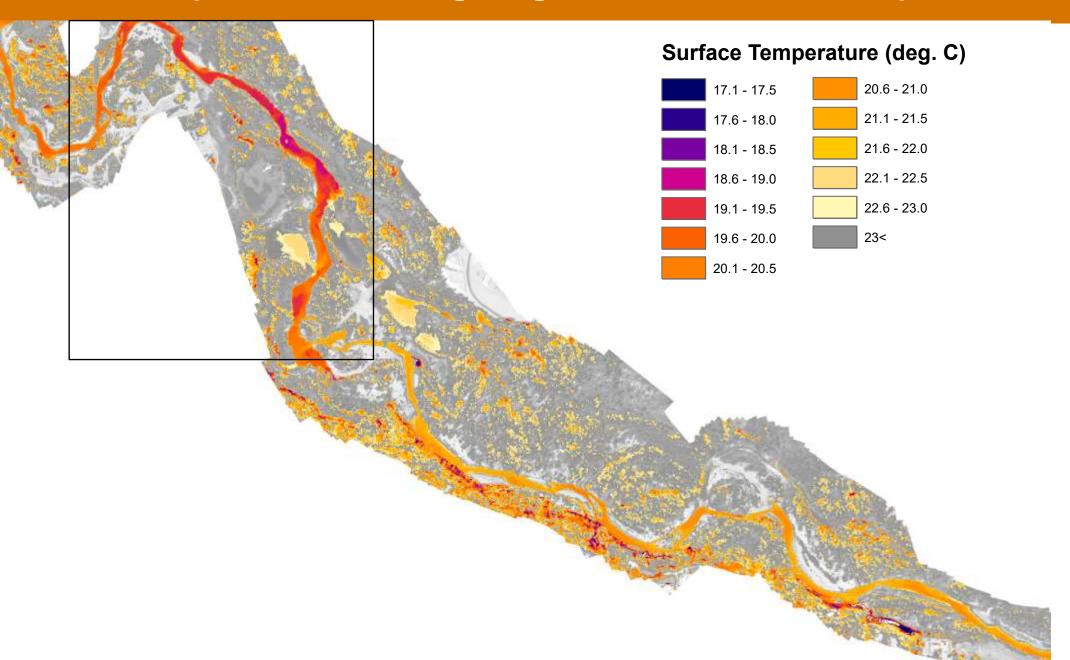


### Water Temperature Modeling - In-situ Temperature Monitoring Results



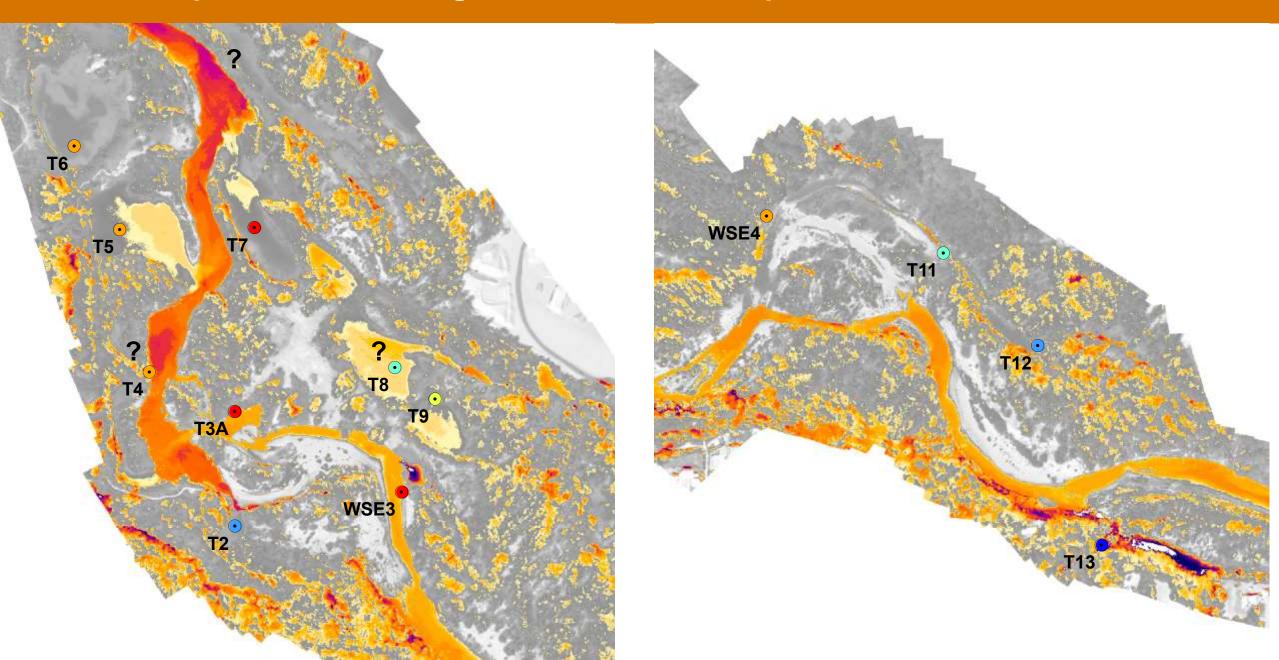


### Water Temperature Modeling - High Resolution FLIR Temperature Survey (2020)





### Water Temperature Modeling – FLIR/In-Situ Comparison



### **Water Temperature Modeling – Sample Results**

Low flow (60 cfs)

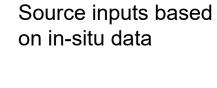
Base flow (500 cfs)





Alt 3



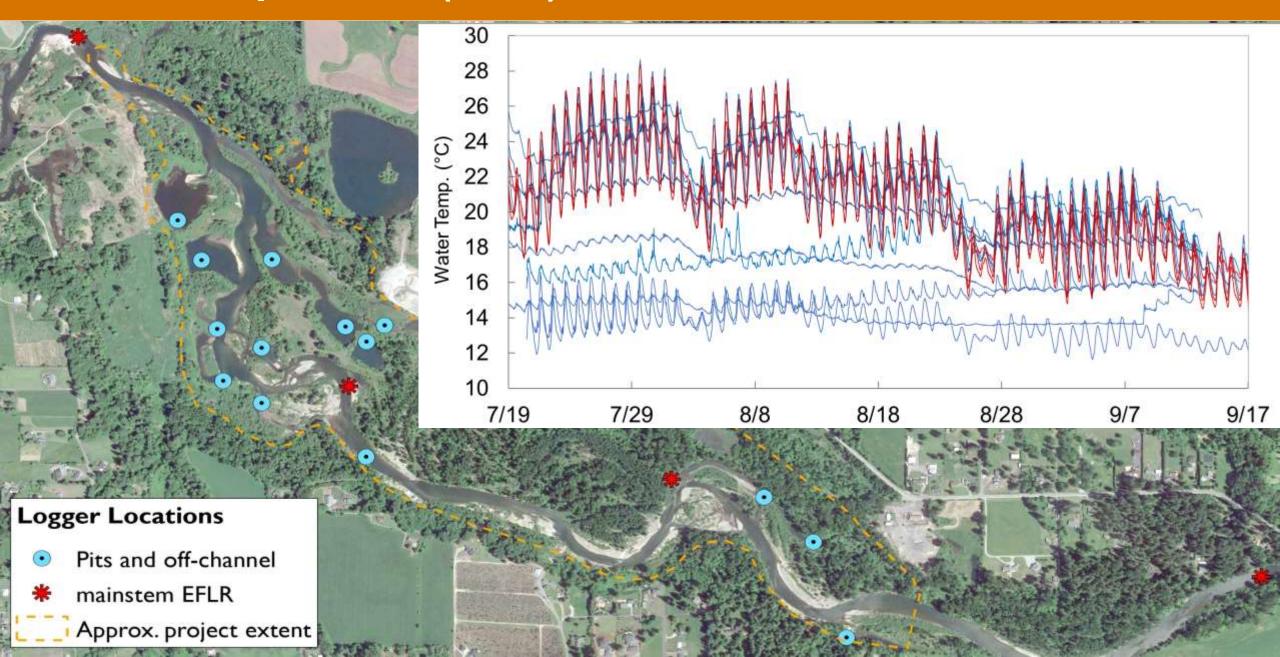




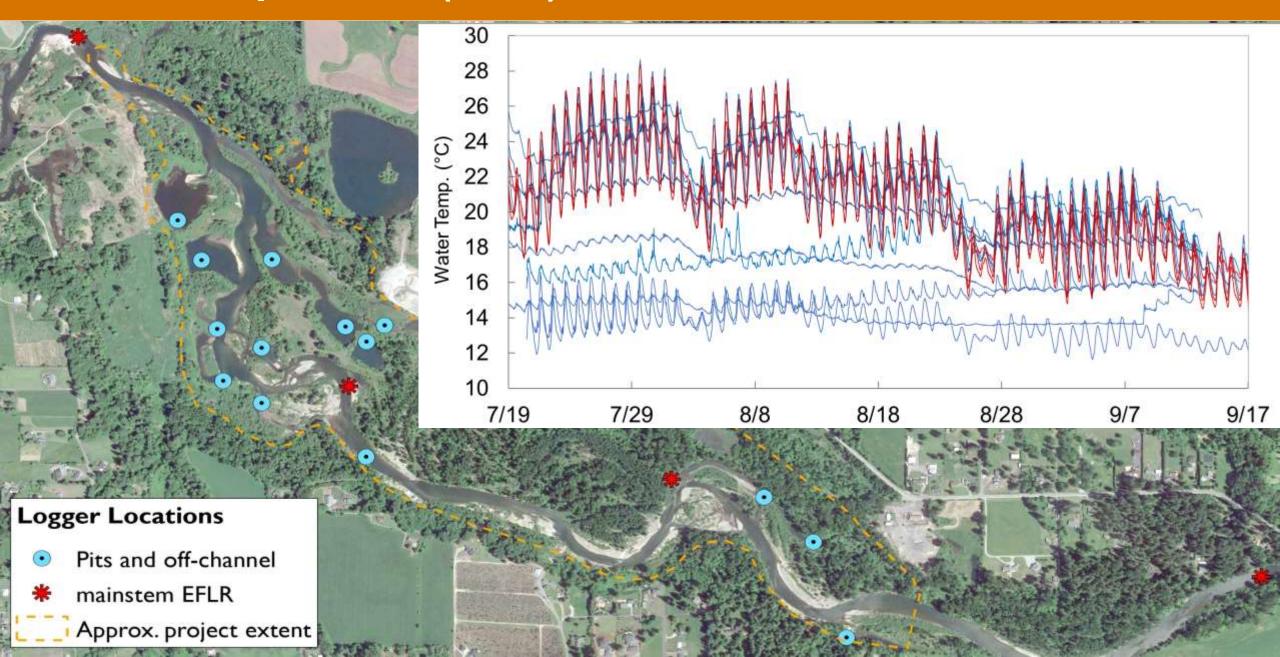




# Water Temperature (2018)



# Water Temperature (2018)



# Water Temperature (2018)

#### Model Next Steps:

#### 1. Hydro/Temperature Model Updates

- refine Alternative 3
  - improve cut/fill balance (currently 400,000 yd^3 deficit)
  - o remove cross-floodplain slope
- create Alternative 2/3 hybrid (2-3 channel option)
  - 2-3 thread system through Pits area
- refine water temperature inputs based on new FLIR data
- begin evaluating Mill Creek and Side Channel alternatives

#### 2. Run morpho-dynamic simulations



